

DESIGN OF THE WASTEWATER TREATMENT PLANT BUILDING IN HULUBEȘTI VILLAGE

Simona-Alexandra SBURĂTURĂ

Scientific Coordinator: Assoc. Prof. PhD. Eng. Augustina Sandina TRONAC

University of Agronomic Sciences and Veterinary Medicine of Bucharest, 59 Mărăști Blvd, District 1, 011464, Bucharest, Romania, Phone: +4021.318.25.64, Fax: + 4021.318.25.67.

Corresponding author email: sburatura.simona93@gmail.com

Abstract

The article refers to the construction of the wastewater treatment plant building from Hulubești village, located in Dâmbovița county, Romania.

The building will be a new rectangular reinforced concrete semi-buried construction. It was adopted the elastic mat foundation type.

For checking the strength of the structure, it has been treated the loads given by: weight, wind, snow, service loads and seismic action. As a result of stability calculations it could be adopted the best solution regarding the building construction.

Key words: stability calculations, loads, building, construction.

INTRODUCTION

Wastewater treatment plant is designed to take waste water from street containers and to change its physio-chemical parameters in order to obtain values permitted by legislation for the evacuation.

Construction of the wastewater treatment plant building does not involve any special requirements in terms of structure. Wastewater treatment plant has both underground and over ground components and it is partially covered with the operational building (S.C. ProMs Concept Group S.R.L., 2012).

The positioning of tanks as well as the aboveground components is given by technological characteristics and conditions of the site.

The wastewater treatment plant building has a rectangular structure in plan and is made of monolith reinforced concrete. The foundation will be of elastic mat.

MATERIALS AND METHODS

The materials used for the construction of the wastewater treatment plant building are the following:

- Autoclaved aerated concrete bricks (B.C.A.), quality class GBN30, GBN20 (SR EN 771-4:2004/A1:2005) 30 cm and 20 cm thickness;
- Concrete for structural strength elements class C25/30 (SR EN 206-1, SR EN 13510, CP012/1-2007);
- Reinforcing bars: longitudinal- steel PC52; transversal- steel OB37.

The following material characteristics are taken into account (Table 1):

Table 1. Materials features

| | | |
|-----------------|---|--------------------------------------|
| CONCRETE C25/30 | Compressive strength of concrete | $R_c = 25 \text{ N/mm}^2$ |
| | Compressive strength calculation for persistent design situations | $R_{c,calc} = 16,667 \text{ N/mm}^2$ |
| | Tensile strength of concrete | $R_t = 1,5 \text{ N/mm}^2$ |
| | Tensile strength calculation of concrete for persistent design situations | $R_{t,calc} = 1 \text{ N/mm}^2$ |
| B.C.A | Specific weight | $Y_c = 6 \text{ kN/m}^3$ |
| STEEL | Feature resistance | $R = 360 \text{ N/mm}^2$ |
| | Strength calculation | $R = 300 \text{ N/mm}^2$ |
| | Elastic modulus | $E = 210000 \text{ N/mm}^2$ |
| | Poisson's ratio | $\nu = 0,3$ |

I considered in the calculations, the following loads (Table 2):

- Weight;
- Snow (SR EN 1991-1-3:2005/NA:2006 TABLE NA1);
- Wind (SR EN 1991-1-4:2006);
- Seismic action (P100/2006);

Table 2. Loads

| | Exterior Masonry | Interior Masonry | Current Floor Plate | Terrace Plate Type |
|----------------------------|-------------------------|------------------|------------------------|-------------------------|
| Weight | 8,576 kN/m | 6,806 kN/m | 2,16 kN/m ² | 2,385 kN/m ² |
| Snow | - | - | - | 1,6 kN/m ² |
| Wind | 0,803 kN/m ² | - | - | 0,803 kN/m ² |
| Seismicity-behavior factor | 2 | 2 | 2 | 2 |

RESULTS AND DISCUSSIONS

The stability calculations are synthetized in Table 3 and demonstrate that the structure checks all the conditions in this regard.

Table 3. Stability verifications

| Test | Equation | Value | Conclusion |
|-------------------------------------|-----------------------------------|-------------|------------------------|
| The pressure on the sole foundation | $p_c = \frac{G_{str}}{A_{as}}$ | 140,805 kPa | Verification satisfied |
| Floatability | $V_{FL} = \frac{G_{str}}{R_{FL}}$ | 5,632 | Verification satisfied |

Reinforcement sizing calculations were done using SCIA ENGINEER software for plate type terrace; the bending moment diagrams in

the X and Y directions are presented below (Figures 1, 2, 3, 4).

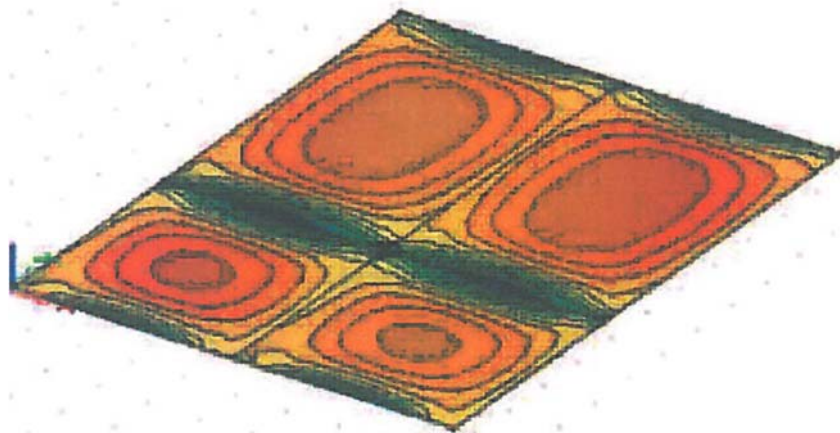


Figure 1. Bending Moment Diagram My- in Plate (Support)

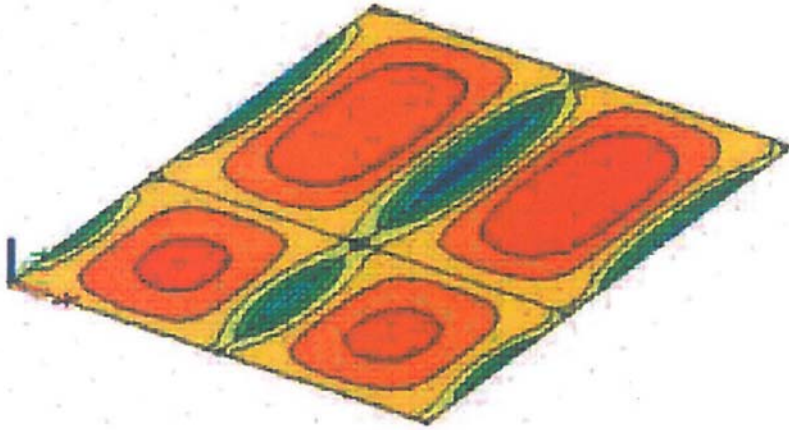


Figure 2. Bending Moment Diagram M_{x-} in Plate (Support)

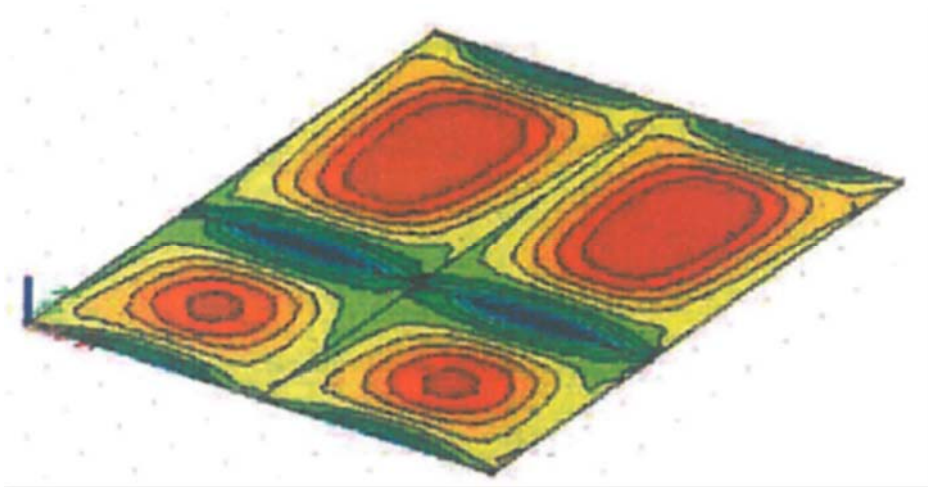


Figure 3. Bending Moment Diagram M_{y+} in Plate (Field)

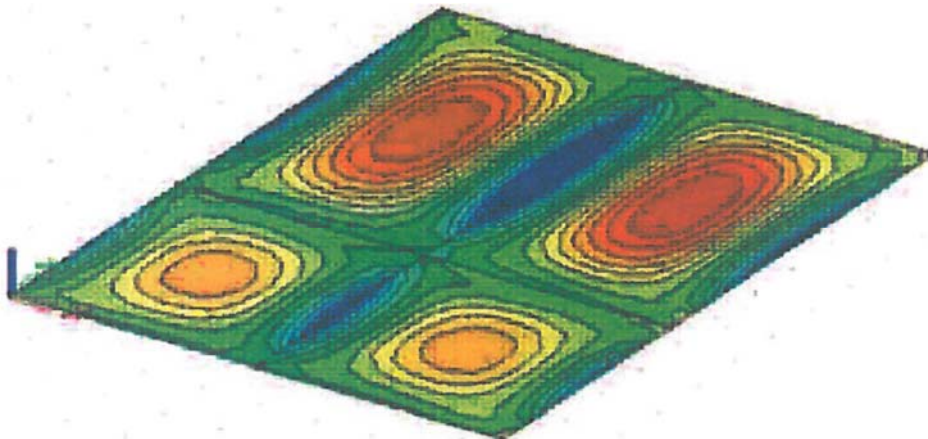


Figure 4. Bending Moment Diagram M_{x+} in Plate (Support)

Structural and calculation elements for the analysed structure are presented below (Table 4).

Table 4. Structural and Calculation Elements

| Element | Value | | | |
|---|-------------|-------------|-------------|-------------|
| | Field | | Support | |
| | Direction X | Direction Y | Direction X | Direction Y |
| Plate thickness, t_p (cm) | 15 | | | |
| Maximum moment M_c (kNm) | 7,67 | 9,81 | 12,25 | 15,115 |
| The concrete cover plate a (mm) | 30 | | | |
| Effective cross-sectional height h_0 (cm) | 12 | | | |
| Bending moment coefficient m_f | 0,032 | 0,041 | 0,051 | 0,063 |
| The ratio between the compression area and the effective area ξ | 0,032 | 0,042 | 0,052 | 0,065 |
| The minimum calculated reinforcement percentage p_{minc} (%) | 0,18 | 0,232 | 0,291 | 0,362 |
| Necessary reinforcement area $A_{a,nec}$ (mm ²) | 216,573 | 278,309 | 349,436 | 433,987 |
| Reinforcement bars diameter ϕ_b (mm) | 8 | 8 | 10 | 10 |
| Bars distance l_b (cm) | 15 | 15 | 15 | 15 |
| Effective reinforcement area A_{ef} (mm ²) | 335,103 | 335,103 | 523,599 | 523,599 |

Calculations and diagrams obtained (Table 4, Figures 1, 2, 3, 4) entitles me to propose the following reinforcement:

- For field- direction X and direction Y- bars ϕ 8/15 cm;
- For support- direction X and direction Y- bars ϕ 10/15 cm.

CONCLUSIONS

1. The treatment plant building is indispensable to achieve the goal of wastewater treatment.
2. The shape and size of the building are imposed by technological processes that are required for wastewater treatment.
3. The verification of stability and strength calculations supports the final solutionproposal for the plant building underlying the actual design.

REFERENCES

- S.C. ProMs Concept Group S.R.L., “Infiintare retea de canalizare si statie de epurare in comunaHulubesti, judetulDambovita”, 2012.
- CP012/1-2007 “CODE OF PRACTICE FOR THE MANUFACTURING OF CONCRETE” P100/2006 “CODE OF SEISMIC DESIGN”
- SR EN 771-4:2004/A1:2005 “Specification for masonry units - Part 4: Autoclaved aerated concrete masonry units”
- SR EN 206-1 “Concrete - Part 1: Specification, performance, production and conformity”
- SR EN 13510 “Earth-moving machinery - Roll-over protective structures - Laboratory tests and performance requirements (ISO 3471:1994, including Amendment 1:1997 modified)”
- SR EN 1992-1-1:2004 “Eurocode 2: Design of concrete structures. Part 1-1: General rules and rules for buildings”
- SR EN 1991-1-3:2005/NA: 2006 “Eurocode 1 - Actions on structures - Part 1-3: General actions - Snow loads. National Annex”, Table NA1
- SR EN 1991-1-4:2006 “Eurocode 1: Actions on structures - Part 1-4: General actions - Wind actions”

SECTION 03
DISASTER MANAGEMENT

